



M42 Junction 9 - 2014 Base Model

Local Model Validation Report

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1 Introduction

Background

- 1.1 JMP Consultants Ltd has been commissioned by the Highways Agency (HA) and Warwickshire County Council (WCC) to develop a fully calibrated and validated 2014 base year model of the M42 Junction 9 and the adjoining local network.
- 1.2 The base model has been developed using the micro-simulation package, S-Paramics and in line with current traffic modelling guidelines. This report provides details of the tasks and methodology undertaken to develop the base model, and provides details on the calibration and validation of the model.

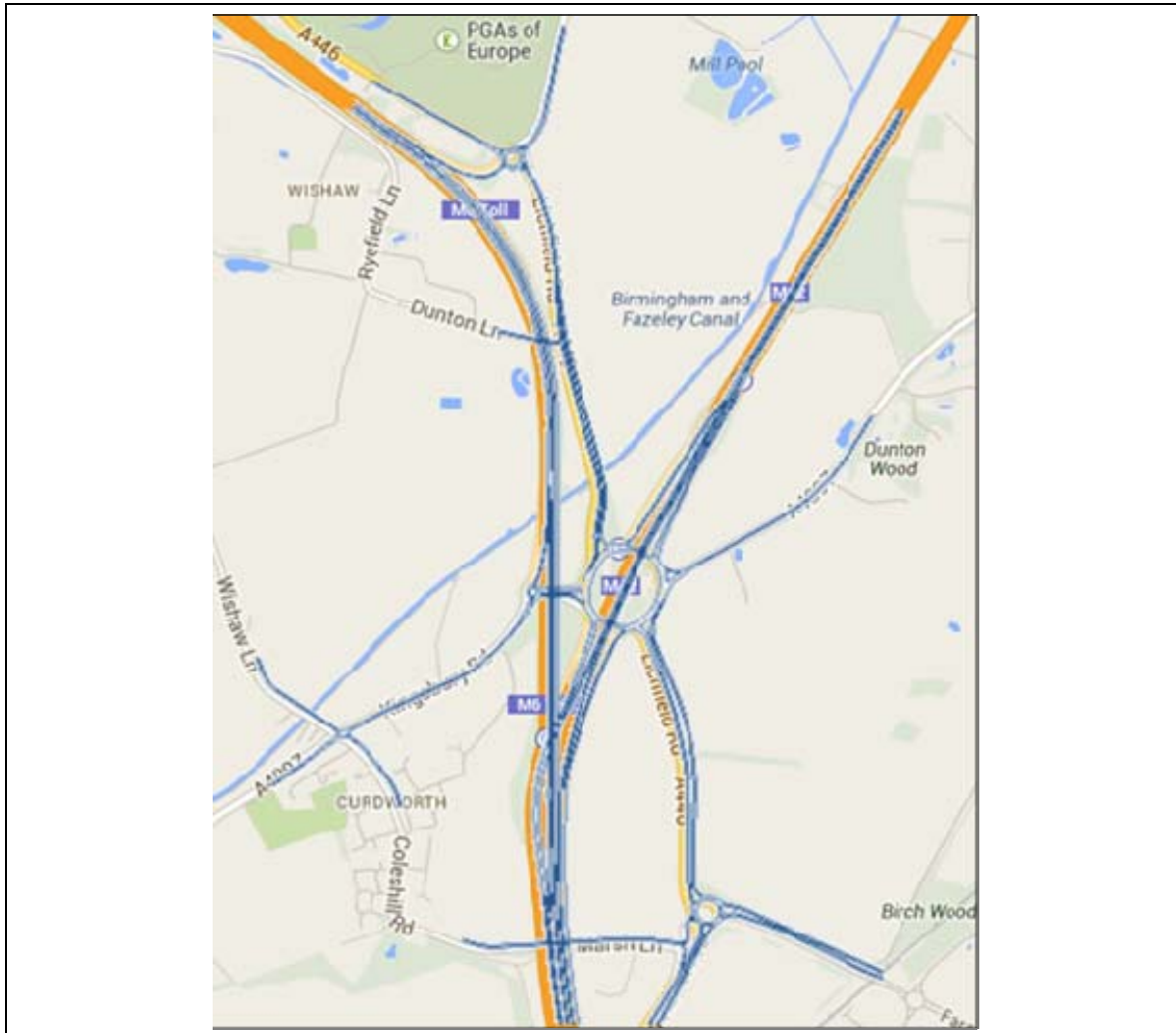
Objectives

- 1.3 The aim of this commission has been to produce a calibrated and validated base model of M42 Junction 9 for the AM (07:00 to 10:00) and PM (16:00 to 19:00) periods.

Model Extent

- 1.4 The network has been developed to cover an area as captured in Figure 1.1 below and includes the following junctions:
 - M42 Junction 9
 - A446 / A4091 / Lichfield Road / M6 Toll
 - A446 / Dunton Lane
 - A4097 / Coleshill Road / Kingsbury Road / Wishaw Lane
 - A446 / Faraday Avenue / Lichfield Road / Marsh Lane

Figure 1.1 M42 Junction 9 2014 Base Model Extent



Purpose of Report

- 1.5 The objective of this report is to summarise all the aspects of the base year model and determine that the model has been calibrated and validated to a level in line with its intended use for the study of future year demand forecasting and scheme assessment.

Report Structure

- 1.6 Chapter 2 of this report includes a summary of the data that was used in the model development process.
- 1.7 Chapter 3 and 4 provide an overview of network inclusions and matrix development process.
- 1.8 Chapter 5 and 6 provide details of the model calibration and validation process and presents the final results.
- 1.9 Chapter 7 provides the summary and conclusions.

2 Survey Data

Data Sources

2.1 A number of data sources have been utilised in the development of the 2014 base model, including:

- Road infrastructure information from digital mapping,
- Manual Classified Count surveys,
- Queue length surveys, and
- Journey time surveys.

Traffic Surveys

2.2 A programme of weekday traffic surveys was carried out by Traffic Survey Partners (TSP) Ltd on Tuesday 25th February 2014, to provide travel demand information for the network. The survey programme is presented in Table 2.1 and the specification summarised below:

- Manual Classified Counts (MCC) – 15 minute count intervals for the AM period 07:00 to 10:00 and PM period 16:00 to 19:00.
- Queue length Surveys – Queues in metres recorded at 15 minute intervals across the AM period 07:30 to 09:30 and PM period 16:30 to 18:30.
- Journey Time Surveys – 16 runs for each route during the AM period 07:30 to 09:30 and the PM period 16:30 to 18:30.

Table 2.1 Survey Programme

Junction / Route	Survey Specification
A446 / Lichfield Rd / Dunton Ln	MCCs & Queue surveys
A446 / Faraday Ave / Lichfield Rd / Marsh Ln	MCCs & Queue surveys
M42 Junction 9	Queue surveys
Kingsbury road. M6 Toll roundabout	Queue surveys
A4097 / Coleshill Road / Kingsbury Road / Wishaw Lane	Queue surveys
A446 / A4091 / Lichfield Rd / M6 Toll Slip	Queue surveys
A446 / A4091 / M6 Toll to A446 / Faraday Ave / Marsh Ln	Journey Time Survey

2.3 Existing MCC data, collected by PCC in November 2013, was also made available by the HA for the following junctions;

- M42 Junction 9
- Kingsbury Road / M6 Toll roundabout
- A4097 / Coleshill Road / Kingsbury Road / Wishaw Lane
- A446 / A4091 / Lichfield Rd / M6 Toll Slip

2.4 ATC data (collected by PCC in November 2013) on Kingsbury Road (west of A4097 / Coleshill Rd / Wishaw Ln) and on A446 Lichfield Road (between Dunton Lane and Junction 9) was also used in model development.

3 NETWORK DEVELOPMENT

Time period

3.1 The following time periods were modelled:

- AM Peak Period: 07:00 – 10:00
- PM Peak Period: 16:00 – 19:00

3.2 S-Paramics version 2011.1 has been used in the development, calibration and validation of the 2014 M42 Junction 9 Base model.

Traffic Signals

3.3 M42 Junction 9 is a signalised roundabout which comprises six arms, with circulating links of up to five lanes. There are no other signals in the model area.

3.4 M42 Junction 9 is Microprocessor Optimised Vehicle Actuation (MOVA) controlled. MOVA optimises the signal timings on the roundabout by meeting the demand requirements and altering green time.

3.5 Fixed signal timings were optimised to represent the MOVA control for the M42 junction 9.

Zone System

3.6 Zones within a traffic model provide loading points where traffic can enter the model. In this model the zone system is defined by route zones only. Each of the roads included in the model has a zone, which is added into the model via a zone connector. Table 3.1 presents details of the modelled zones.

Table 3.1 Model Zones

Zone Number	Description
1	M6 Toll (North)
2	A446 Lichfield Road (North)
3	A4091
4	M42 (North of J9)
5	A4097 Kingsbury Road (East)
6	Faraday Avenue
7	A446 Lichfield Road (South)
8	Marsh Lane
9	M42 (South of J9)
10	Coleshill Road
11	A4097 Kingsbury Road (W)
12	Wishaw Lane
13	Dunton Lane
14	M42 (South of J9) [left lane]

Link Classification

- 3.7 From the outset a detailed set of road class categories were identified to form a sensible routing pattern in case the model would be expanded to include route choice in the future.
- 3.8 The first level of route definition is the distinction between major and minor links. A major link is categorised as a signposted or heavily trafficked route.
- 3.9 There are four main link categories as follows:
- 30mph minor route, cost of 1.0; Minor or Unclassified Access Roads
 - 40mph major route, cost of 1.2; Local Distributors such as 'B Roads'
 - 50-60mph major route, cost of 1.0; Primary Signposted Routes Such as 'A Roads'
 - 70mph major route, cost of 0.7; Major Signposted Routes such as Motorways
- 3.10 The model seeks to replicate the route hierarchy as seen on the ground where roads are classified by purpose and their regional importance.
- 3.11 All links have been coded to the operating carriageway width and speed limits as described on the OS plan and derived from online mapping.
- 3.12 In summary, the 'physical' network in the model has been calibrated for the model area and reflects existing network conditions in February 2014 in line with the available data and data gathered during recent surveys.

Junction Calibration

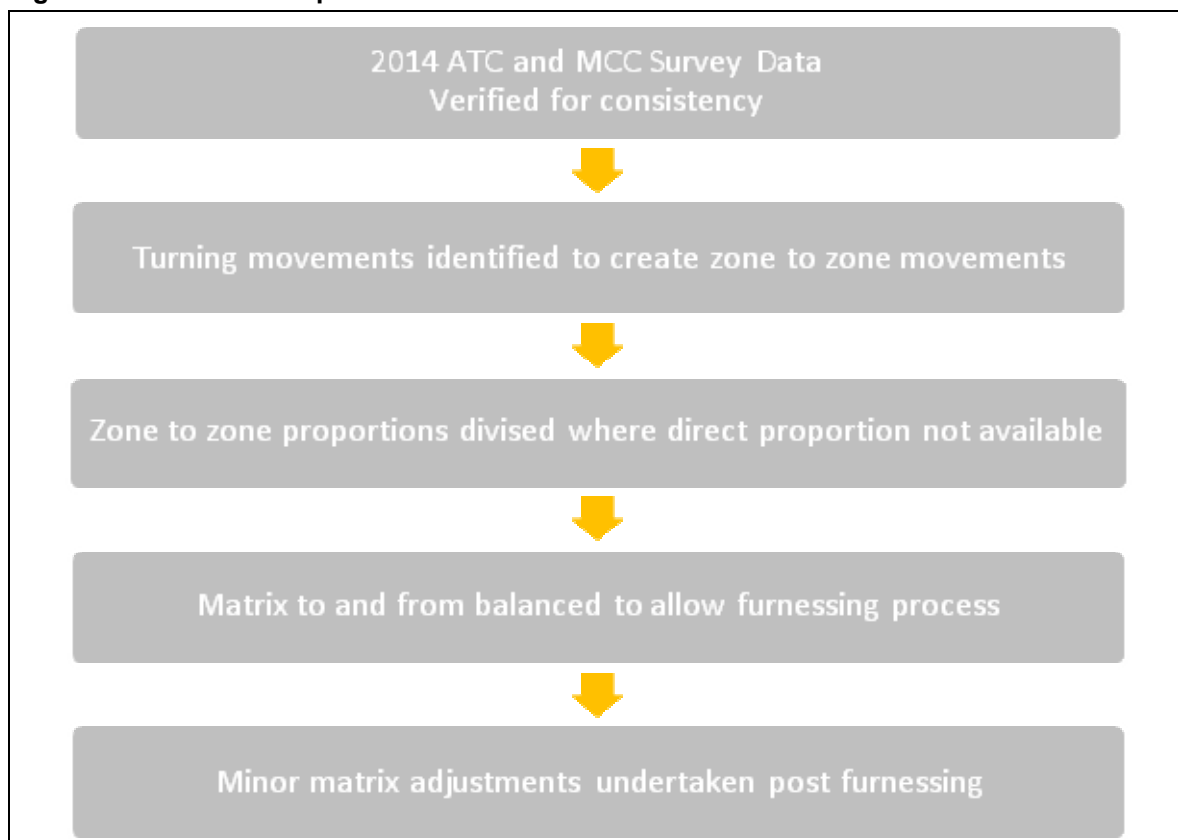
- 3.13 Junction modifiers were applied throughout the network to allow the replication of vehicle behaviour and traffic throughput at the primary junctions and roundabouts in the network. Junction calibration parameters are included in S-Paramics to allow reflection of observed traffic flow and queue levels at individual junctions.
- 3.14 A standard 10m visibility value was initially applied at all roundabout approach arms in the network. During the calibration process, junction visibility values and gap acceptance values (lane merge / lane cross / path cross) were revised, to allow the model to replicate the junction capacity and vehicles queuing levels observed.
- 3.15 Vehicle maximum speeds were reduced from the default values to 75mph for cars and LGVs, and to 65mph for OGV1 and OGV2 vehicle types.

4 TRIP MATRIX DEVELOPMENT

Introduction

- 4.1 The development of the 2014 trip matrices utilised the relevant information obtained during the data collection exercise. Matrices were derived from the surveyed turn and link count data, with additional data for the M42 and M6 Toll mainline informed by the TRADS database. The detailed methodology to create the demand matrices is discussed in this section.
- 4.2 Figure 4.1 provides a flow chart representing the basic procedures followed to develop the prior matrices and then refine the model's calibration and routing.

Figure 4.1 Matrix Development Flow Chart



Matrix Construction

- 4.3 Once the survey data had been checked for consistency, separate light and heavy turn counts were used to create prior matrices. Working from the origin zone the first turn count was multiplied by the percentage making the next turn and so on until the destination zone.
- 4.4 The prior matrices were then used to check the totals to and from zones against link counts. There were slight differences between row and column totals with the link counts so the matrices were furnished. The furnished matrices were used in the base model as the initial demand matrices.
- 4.5 A summary of the total derived trip matrices is provided in the tables below.

Table 4.1 AM Demand Matrix Totals

	07:00 – 08:00	08:00 – 09:00	09:00 – 10:00	Period Total
Matrix 1 – Lights	12,592	12,418	9,539	34,549
Matrix 2 - Heavies	2,486	2,238	2,189	6,913
TOTAL	15,078	14,656	11,728	41,462

Table 4.2 PM Demand Matrix Totals

	16:00 – 17:00	17:00 – 18:00	18:00 – 19:00	Period Total
Matrix 1 – Lights	12,925	13,499	10,535	36,959
Matrix 2 - Heavies	2,114	1,711	1,427	5,252
TOTAL	15,039	15,210	11,962	42,211

Demand Profiles

- 4.6 The release rate of traffic leaving each zone will vary throughout the model period. This is simulated by applying specific demand profiles to each zone.
- 4.7 The junction turn count data was evaluated to obtain the profiles over the modelled periods, 07:00 to 10:00 and 16:00 to 19:00, for both light and heavy vehicles.
- 4.8 In most cases a release profile was derived directly from the downstream junction approaches traffic volumes. In other cases, where this data was not available, a proxy profile was derived from surrounding locations. Separate light and heavy profiles were developed in this way.

5 MODEL CALIBRATION

Introduction

- 5.1 The model calibration process was carried out using the criteria specified in DMRB, Volume 12, Section 2, Part 1: Traffic Appraisal in Urban Areas. DMRB Volume 12 suggests individual link flows should have a GEH less than 5 in 85% of cases over a one hour interval.
- 5.2 The 2014 M42 Junction 9 Base model has been calibrated to vehicle link flows, in line with DMRB criteria, which have been derived from the surveyed turning counts. Turning counts and queue lengths have also been compared to observed data as part of the calibration process.

Calibration Criteria

- 5.3 The model calibration process has been carried out in accordance with the DMRB guidelines, summarised in Table 5.1.

Table 5.1 DMRB Guideline Model Acceptability Criteria

Criteria and Measure	Acceptability
Assigned Hourly Flows	
Individual flows within 100vph (flows<700vph)	85% of all cases
Individual flows within 15% (flows 700-2700vph)	85% of all cases
Individual flows within 400vph (flows>2700vph)	85% of all cases
GEH statistic: individual flows GEH<5	85% of all cases
Modelled Journey Times	
Times within 15% (or 1 minute, if higher)	85% of all cases

- 5.4 The GEH statistic is used during the calibration of a model to compare the difference between an observed flow and a modelled flow, and is defined as shown below:

$$GEH = \sqrt{\frac{2(\text{Modelled} - \text{Observed})^2}{(\text{Modelled} + \text{Observed})}}$$

- 5.5 The GEH statistic is used in preference to the absolute or relative flow difference as it can cope with a wide range of traffic flows. Where an absolute difference of 100 PCUs/hr can be important in a flow of 200 PCUs/hr it is largely irrelevant in a flow of several thousand PCUs/hr.

Model Stability

- 5.6 S-Paramics is not an equilibrium modelling software and is subject to minor variations in the assignment from one run to the next. This being the case, it is normal to use the mean link flow values derived from the multiple runs. The 2014 M42 Junction 9 Base model has been calibrated using 10 log runs. Due to the Base model not having route choice the stability assessment found that 3 model runs would have been sufficient for this model.
- 5.7 Full details of the stability assessment are given in Appendix A.

Traffic Flow Comparisons

DMRB Flow Criteria

- 5.8 DMRB Vol. 12 recommends comparisons with observed flows at an hourly level so each modelled hour has been assessed individually. Total flows across the 3 hours have been combined and assessed against the same criteria as an additional check.
- 5.9 The percentage of link and turn count locations meeting the DMRB criteria are summarised in the tables below. A total of 48 link flows and 69 turns have been assessed.

Table 5.2 Link Flow Calibration

	AM				PM			
	07 - 08:00	08 - 09:00	09 - 10:00	07 - 10:00	16 - 17:00	17 - 18:00	18 - 19:00	16 - 19:00
Obs <700vph	23	20	26	10	23	22	30	10
Mod within 100vph	23	20	26	9	23	22	29	10
% within DMRB	100%	100%	100%	90%	100%	100%	97%	100%
Pass/Fail	Pass	Pass	Pass	Pass	Pass	Pass	Pass	Pass
Obs 700 to 2700vph	24	28	22	24	24	25	18	22
Mod within 15%	24	28	22	24	24	24	18	22
% within DMRB	100%	100%	100%	100%	100%	96%	100%	100%
Pass/Fail	-	Pass	Fail	Pass	Pass	Pass	Pass	Pass
Obs > 2700vph	1	0	0	14	1	1	0	16
Mod within 400vph	1	0	0	14	1	1	0	16
% within DMRB	100%	-	-	100%	100%	100%	-	100%
Pass/Fail	Pass	Pass	Pass	Pass	Pass	Pass	Pass	Pass

Table 5.3 Turn Flow Calibration

	AM				PM			
	07 - 08:00	08 - 09:00	09 - 10:00	07 - 10:00	16 - 17:00	17 - 18:00	18 - 19:00	16 - 19:00
Obs <700vph	62	61	65	50	62	62	65	47
Mod within 100vph	62	61	65	49	62	62	64	46
% within DMRB	100%	100%	100%	98%	100%	100%	98%	98%
Pass/Fail	Pass	Pass	Pass	Pass	Pass	Pass	Pass	Pass
Obs 700 to 2700vph	7	8	4	16	7	7	4	18
Mod within 15%	6	8	4	16	7	7	3	17
% within DMRB	86%	100%	100%	100%	100%	100%	75%	94%
Pass/Fail	Pass	Pass	Pass	Pass	Pass	Pass	Fail	Pass
Obs > 2700vph	0	0	0	3	0	0	0	4
Mod within 400vph	0	0	0	2	0	0	0	4
% within DMRB	-	-	-	67%	-	-	-	100%
Pass/Fail	Pass	Pass	Pass	Fail	Pass	Pass	Pass	Pass

- 5.10 As more than 85% of the link flows meet DMRB's flow criteria for each simulated hour (and the 3 hour period as a whole), the Base model can be said to calibrate well to traffic flows.
- 5.11 An assessment of the turn flows against DMRBs criteria for flows indicates a very high level of calibration with only 7 turn flows out of a total of 552 fail to meet DMRB's criteria.

DMRB GEH Assessments

- 5.12 DMRB recommends that in more than 85% of cases the individual link flows should have a GEH value of less than five.
- 5.13 Table 5.4 and Table 5.5 present the percentage of link flow sites by GEH value for each modelled hour. For completeness Table 5.6 and Table 5.7 present the equivalent assessment based on the turn counts.

Table 5.4 AM Link Flow Comparison – GEH

	07:00 - 08:00		08:00 - 09:00		09:00 - 10:00	
Link Counts	48		48		48	
GEH ≤ 5	47		47		48	
%	98%		98%		100%	
GEH ≤						
6	47	98%	47	98%	48	100%
7	48	100%	48	100%	48	100%

Table 5.5 PM Link Flow Comparison – GEH

	16:00 - 17:00		17:00 - 18:00		18:00 - 19:00	
Link Counts	48		48		48	
GEH ≤ 5	48		47		48	
%	100%		98%		100%	
GEH ≤						
6	48	100%	48	100%	48	100%
7	48	100%	48	100%	48	100%

Table 5.6 AM Turn Flow Comparison – GEH

	07:00 - 08:00		08:00 - 09:00		09:00 - 10:00	
Turn Counts	69		69		69	
GEH ≤ 5	67		68		69	
%	97%		99%		100%	
GEH ≤						
6	67	97%	68	99%	69	100%
7	67	97%	68	99%	69	100%
8	68	99%	69	100%	69	100%
9	68	99%	69	100%	69	100%
10	68	99%	69	100%	69	100%

Table 5.7 PM Turn Flow Comparison – GEH

	16:00 - 17:00		17:00 - 18:00		18:00 - 19:00	
Turn Counts	69		69		69	
GEH ≤ 5	69		69		69	
%	100%		100%		100%	
GEH ≤						
6	69	100%	69	100%	69	100%
7	69	100%	69	100%	69	100%
8	69	100%	69	100%	69	100%
9	69	100%	69	100%	69	100%
10	69	100%	69	100%	69	100%

- 5.14 The tables above show that in the AM and PM periods, the percentage of links and turns with a GEH of less than 5 is greater than 85% in all periods. At least 98% of all link count locations have a GEH value of less than 5 and 97% of all turn counts. The Base model therefore meets the DMRB criteria and confirms that the model calibrates well.
- 5.15 Appendix B presents the link flow locations on a schematic of the network showing the GEH values and DMRBs flow criteria results for each peak hour. A full copy of the GEH assessments for all modelled hours for both links and turns is also included in Appendix B.

Correlation Analysis

- 5.16 A further form of comparison detailed in DMRB that can be used to demonstrate a 'goodness of model fit' is to undertake a correlation analysis between the modelled and observed flow data. This was undertaken for each modelled period using an X-Y scatter plot.
- 5.17 The X-Y scatter plot is a good way of presenting the variation in data in a pictorial format and illustrates the relationship between the observed and modelled turn counts in the model. AM and PM Scatter plots are presented in below.

Figure 5.1 Correlation Analysis: Modelled vs Observed Turn Counts (AM Period 07-10:00)

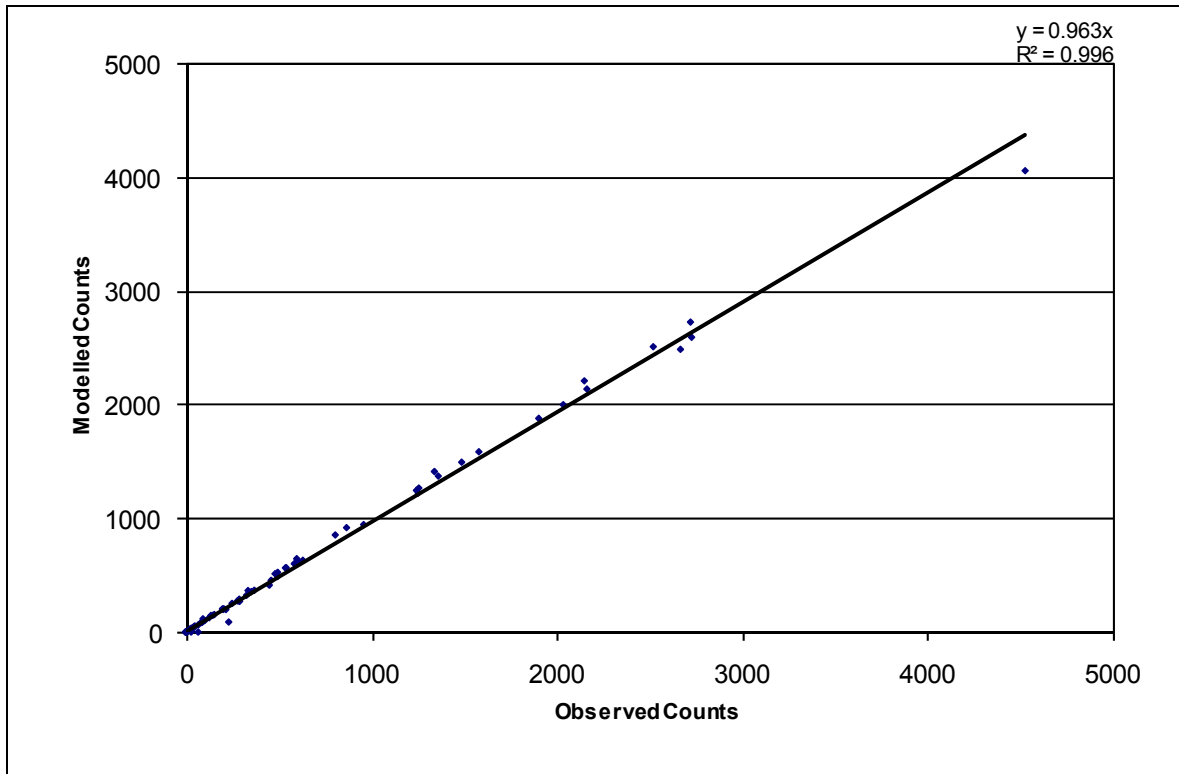
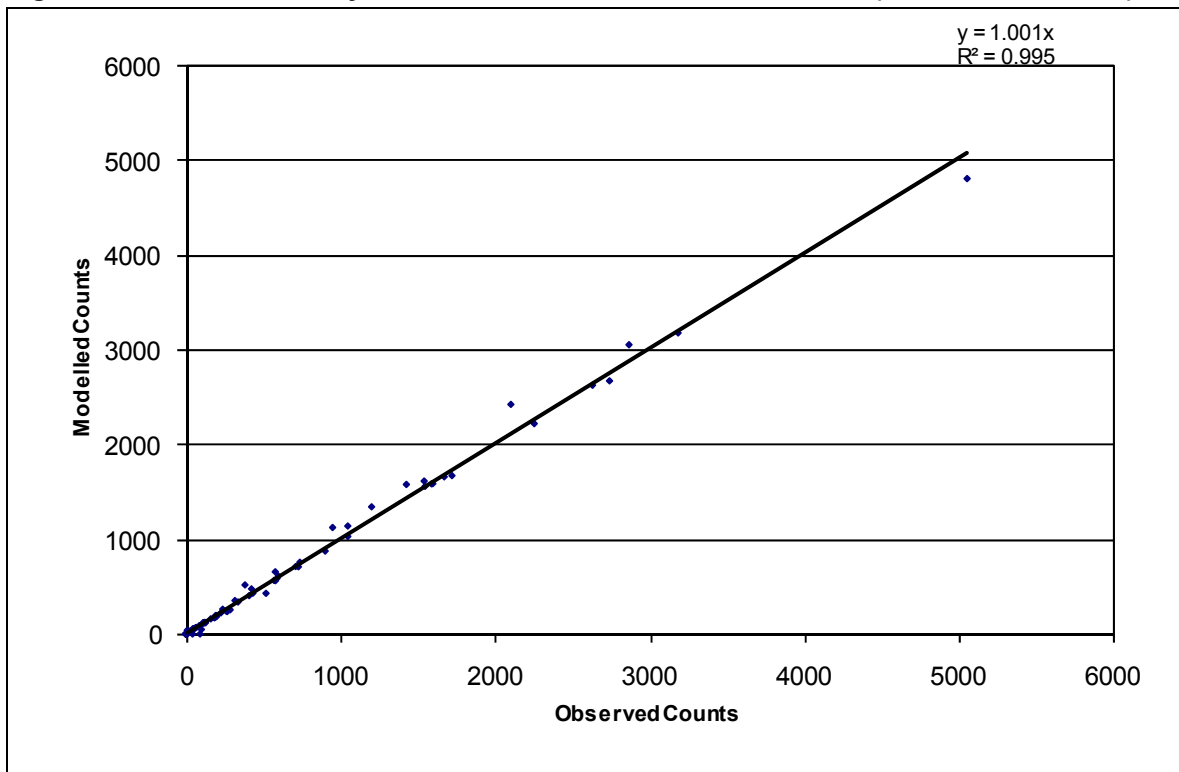


Figure 5.2 Correlation Analysis: Modelled vs Observed Turn Counts (PM Period 16-19:00)



- 5.18 The linear line of best fit through the data was plotted for comparative purposes. The closer the line of best fit is to the $Y=X$ line, the closer the relationship between each of the observed and modelled flows.
- 5.19 The slope of the best-fit line through the origin for the modelled data provides an indication of whether modelled values are globally over or under estimated compared to the observed flows. DMRB suggests that acceptable gradient values for this line should be between 0.9 and 1.10. The line of best fit for the AM and PM modelled period are as follows:
- AM Period 07:00 – 10:00; $y = 0.9634x$
 - PM Period 07:00 – 10:00; $y = 1.0019x$
- 5.20 As such, the gradient values satisfy the DMRB criteria.
- 5.21 In addition, the correlation coefficient (R) gives a measure of the 'goodness' of model fit. DMRB suggest that values of R should be above 0.95. For this reason, the coefficient of determination (R^2) was calculated for each modelled period to present the variability in the data set. This would suggest that R^2 values above 0.9025 (are within the acceptability guidelines set out in DMRB).
- 5.22 The correlation analysis shows an R^2 for the AM and PM modelled period as follows:
- AM period 07:00 – 10:00; $R^2 = 0.996$
 - PM period 07:00 – 10:00; $R^2 = 0.995$
- 5.23 Both the AM and PM modelled periods therefore satisfy the DMRB criteria.
- 5.24 In a perfect match, the observed and assigned flows plotted would form a single line and show a positive correlation between each variable, i.e. the line of best fit would be $Y=X$. Given that traffic flows vary on a day to day basis and that modelling generally aims to simulate an average day then this realistically can never be achieved.
- 5.25 The results nonetheless show that for all periods the line of best fit closely matches the line $Y=X$. The results show a close relationship between the observed flows and those assigned within the model and a good level of calibration has been achieved.

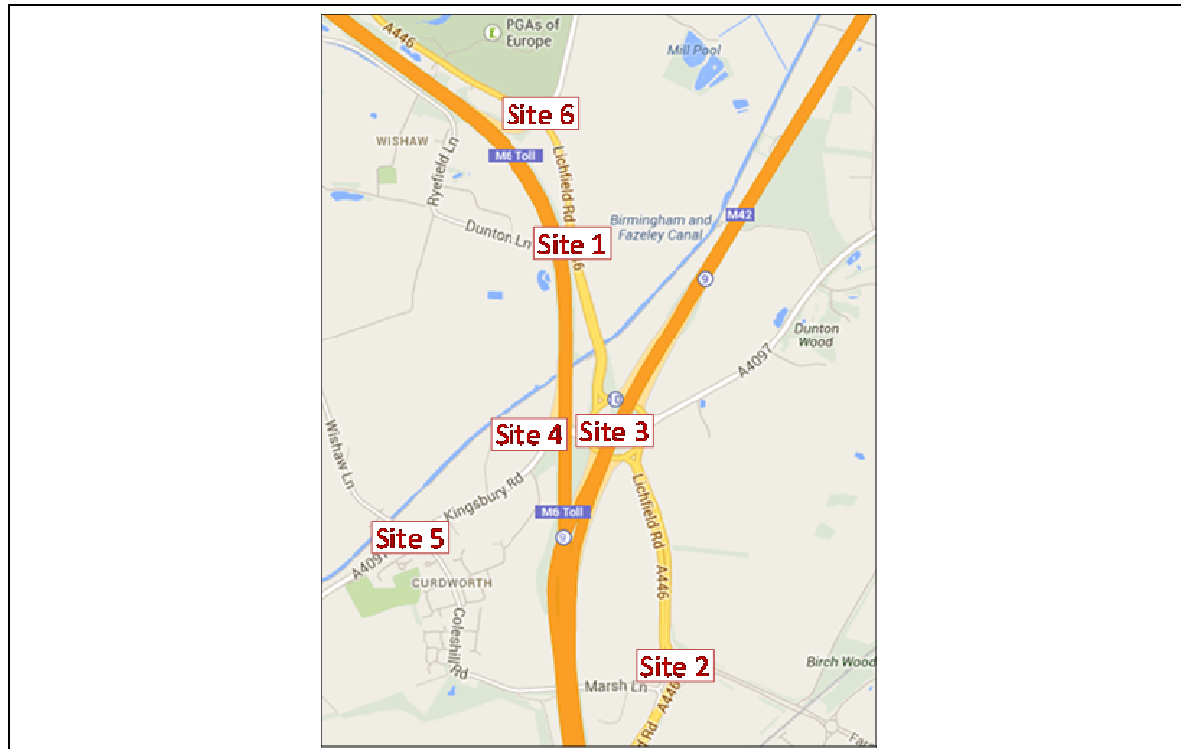
Queue lengths

- 5.26 Queue lengths have been used to calibrate the Base model. DMRB does not specify criteria for queues, but this is included to show how the model compares to observed data.
- 5.27 Queue data was collected from 07:30 to 09:30 and 16:30 to 18:30 and the queue length observed per lane was recorded. The maximum queue length (metres) over all lanes was used to calibrate the Base model.
- 5.28 The mean maximum queue length (based on the average of the hour's 15 minute maximums) for the AM and PM peak hour is presented in Table 5.8 and Figure 5.3 highlights the site locations.
- 5.29 The queue analysis for the full survived period (i.e. including the 30 minutes either side of the peak hours) is provided in Appendix C.

Table 5.8 Maximum Queue Length Comparison (Peak Hours)

Site	Approach	0800-0900				1700-1800			
		Obs (m)	Mod (m)	Difference		Obs (m)	Mod (m)	Difference	
				m	vehs			m	vehs
Site 1	Dunton Ln	5	0	-5	-1	3	6	3	1
Site 2	Faraday Ave	68	49	-19	-3	54	160	106	18
	A446 (S)	21	26	5	1	38	50	13	2
	Marsh Ln	16	30	14	2	15	29	14	2
	A446 (N)	88	61	-27	-4	38	36	-1	0
Site 3	M42 (N)	49	63	14	2	45	63	18	3
	Kingsbury Rd (E)	51	85	34	6	38	62	24	4
	Lichfield Rd (S)	80	100	20	3	73	121	49	8
	M42 (S)	65	90	25	4	170	216	46	8
	Kingsbury Rd (W)	70	124	54	9	69	89	20	3
	Lichfield Rd (N)	201	191	-11	-2	83	201	119	20
Site 4	Kingsbury Rd (E)	0	22	22	4	0	16	16	3
	Kingsbury Rd (W)	11	28	16	3	36	24	-12	-2
Site 5	Kingsbury Rd (E)	6	7	1	0	25	18	-7	-1
	Coleshill Rd	13	11	-1	0	26	18	-8	-1
	Kingsbury Rd (W)	11	4	-7	-1	8	8	0	0
	Wishaw Ln	60	34	-26	-4	16	15	-2	0
Site 6	A4091	58	22	-36	-6	28	15	-12	-2
	Lichfield Rd (S)	38	27	-10	-2	25	35	10	2
	M6 diverge	16	25	9	1	13	24	11	2
	Lichfield Rd (N)	84	52	-32	-5	64	74	11	2

Figure 5.3 Queue Assessment Locations



- 5.30 From the comparison of modelled and observed queue lengths the 2014 Base model is shown to calibrate well to queues.
- 5.31 There are two locations where the queue length is significantly higher in the model compared to observed lengths. This occurs in the PM peak hour at the Lichfield Road (North) southbound approach to M42 Junction 9 and on the Faraday Avenue approach to Marsh Lane / Lichfield Road roundabout.
- 5.32 The link counts and queues calibrate well upstream and downstream of these locations as do the journey times on these sections. For this reason, and due to the known difficulties in using queue data in model development, it was deemed inappropriate to further refine the model network to replicate more closely the lengths recorded in the on street survey.

6 MODEL VALIDATION

Introduction

6.1 The 2014 Base model was validated to journey times that were independent to the calibration process. The validation is an independent check of the calibrated model.

Journey Time Comparisons

6.2 Journey time surveys were undertaken during the peak periods for two key routes within the model area as shown in Figure 6.1. Up to 10 journey time runs were undertaken for each route, in each direction, and for each peak period.

6.3 Journey times can be defined in S-Paramics and extracted over the modelled periods. Based on a series of 10 model runs, the journey times have been compared against observed data.

6.4 Sub-routes within each corridor were identified to match the observed journey time timing points. This was intended to enable more accurate assessment of the delay on the specific sections of the network.

Figure 6.1 Journey Paths



- 6.5 DMRB acceptability guidelines recommend that over a journey time route the modelled mean comparison should be within 1 minute or 15% of the observed, in 85% of cases.
- 6.6 Table 6.1 provides a summary of the AM Peak hour assessment of each individual route and the journey time corridor as a whole. The equivalent PM Peak hour data is summarised in Table 6.2.
- 6.7 The journey time comparisons have been plotted over time and are presented in Appendix D.

Table 6.1 Journey Time Validation (AM Peak: 08-09:00)

	Observed (hh:mm)			Modelled (hh:mm)			Difference		DMRB?	
	Min	Mean	Max	Min	Mean	Max	hh:mm	%		
Northbound_1	00:49	01:20	01:59	00:32	01:07	02:02	00:13	-16%	Yes	✓
Northbound_2	00:24	00:46	01:02	00:26	00:44	01:05	00:03	-6%	Yes	✓
Northbound_3	01:02	01:15	01:53	00:48	00:51	01:04	00:24	-32%	Yes	✓
Northbound	02:46	03:21	04:04	02:14	02:57	03:40	00:25	-12%	Yes	✓
Southbound_1	01:06	02:52	05:45	00:55	01:30	02:23	01:22	-48%	No	✗
Southbound_2	00:27	00:47	01:10	00:20	00:35	01:04	00:12	-26%	Yes	✓
Southbound_3	00:46	00:51	00:57	00:35	00:40	00:56	00:11	-22%	Yes	✓
Southbound	02:33	04:31	07:45	02:01	02:48	03:46	01:43	-38%	No	✗

Table 6.2 Journey Time Validation (PM Peak: 17-18:00)

	Observed (hh:mm)			Modelled (hh:mm)			Difference		DMRB?	
	Min	Mean	Max	Min	Mean	Max	hh:mm	%		
Northbound_1	00:55	01:10	01:51	00:32	00:51	01:27	00:19	-27%	Yes	✓
Northbound_2	00:36	01:08	02:03	00:24	00:58	01:33	00:10	-14%	Yes	✓
Northbound_3	01:14	01:33	02:22	01:53	02:30	03:09	00:57	62%	Yes	✓
Northbound	02:51	03:50	05:05	03:28	04:22	05:14	00:31	14%	Yes	✓
Southbound_1	01:04	01:33	02:34	00:58	01:36	02:46	00:03	3%	Yes	✓
Southbound_2	00:23	00:39	01:02	00:21	00:36	01:01	00:04	-9%	Yes	✓
Southbound_3	00:45	00:50	00:55	00:35	00:38	00:46	00:12	-24%	Yes	✓
Southbound	02:21	03:02	04:06	02:07	02:53	03:54	00:09	-5%	Yes	✓

- 6.8 Table 6.1 and Table 6.2 show that for all six journey time sub-routes 5 of the six calibrate to DMRB criteria in the AM (83%) and all of the journey times calibrate to DMRB criteria in the PM (100%).
- 6.9 To improve the AM validation the Lichfield Road southbound queue to M42 Junction 9 would have to increase between 08:00 and 09:00. As it stands the queue assessment indicates the model reflects queues on this approach in line with those observed during the site surveys.

7 Summary and Conclusions

Summary

- 7.1 The 2014 M42 Junction 9 Base model was jointly commissioned by the Highways Agency and Warwickshire County Council in January 2014 and covers the M42 Junction 9 / M6 Toll interchange; A4091 from its junction with A4091 in the north to Faraday Avenue junction to the south; and the A4097 / Wishaw Lane / Coleshill Road to the west. The 2014 Base model has been calibrated and validated using S-Paramics micro-simulation modelling software, version 2011.1.
- 7.2 Traffic surveys were conducted in February 2014 and observed traffic junction turn counts, queue lengths and journey times were recorded. The Base model has been developed to capture 3 hour AM (0700-1000) and 3 hour PM (1600-1900) hours.
- 7.3 The observed turn counts were used to derive a prior matrix that was then furnished to give the Base demand matrices. The count data was also used to create release profiles applied to the modelled zones.
- 7.4 The model was calibrated to observed flows and queue lengths. For each of the AM modelled hours 100% of the modelled links met the DMRB flow criteria, and 98% of the links in the PM hours. The model also calibrated well to the DMRB GEH criteria with a minimum of 98% of all modelled hourly link flows, and 97% of turn counts, having a GEH value of less than 5.
- 7.5 In the AM peak period the modelled queues show a good match to the observed queues at all 21 junction approaches. In the PM peak hour 2 of the 21 sites had higher than observed queue lengths, namely on Faraday Avenue at the south of the model and at Lichfield Road (N) on the approach to the M42 Junction 9. However, calibration checks and journey time assessments provided confidence that the model was still realistic at these locations.
- 7.6 The Base model was validated to observed journey times. In the AM period the modelled journey time southbound along Lichfield Road to M42 Junction 9 was faster than the observed and does meet DMRB criteria at this location only. The modelled queue on the Lichfield (N) approach to M42 Junction 9 was a very close fit to the observed data which would no longer be the case if the journey time on Lichfield Road southbound was to be slowed down to achieve a closer journey time validation. The PM modelled journey times all meet the DMRB criteria.

Conclusions

This report has been produced to detail the 2014 M42 Junction 9 Base model development, and present the calibration and validation results.

Using detailed road network and traffic count data, the micro-simulation model has been successfully developed to reproduce traffic conditions for a typical weekday. The model calibrates well to the observed data and meets DMRB acceptability guidelines.

The model validation demonstrates that the observed and modelled journey times are consistent in terms of average journey times and the variation in journey time across multiple runs throughout the modelled peak periods.

It is therefore considered that this model is fit for considering future year forecasting, assessing any network enhancements, and for use in any future year testing.

Appendix A

Model Stability Assessment

The stability of a model is important when interpreting the results from a model as it ensures a stable average is achieved in terms of travel cost and flows. This also informs the user how many runs should be undertaken to meet the criteria as set out in DMRB Volume 12 Section 2 (pages 4/23 to 4/25 and Appendix H, particularly Appendix H2).

The objective of this assessment is to quantify the number of runs that should be undertaken to ensure a robust output is achieved.

The model was run 10 times in each peak period to undertake the model stability assessment. The results are presented in the tables below.

Table AppA.1 AM Model Stability

Comparison (# of runs)	AAD	RAAD	%FLOW	STDEV	Delta	V
3	0.48	0.05	100	82.35	0.01	0.00
4	0.45	0.03	100	89.54	0.01	0.02
5	0.31	0.02	100	97.06	0.01	0.02
6	0.25	0.02	100	96.73	0.01	0.03
7	0.23	0.02	100	98.69	0.01	0.07
8	0.18	0.02	100	98.37	0.00	0.04
9	0.18	0.01	100	100	0.01	0.03
10	0.14	0.01	100	100	0.00	0.00

Table AppA.2 PM Model Stability

Comparison (# of runs)	AAD	RAAD	%FLOW	STDEV	Delta	V
3	0.51	0.04	100	72.22	0.01	0.40
4	0.34	0.04	100	95.42	0.00	0.54
5	0.33	0.03	100	98.04	0.00	0.75
6	0.25	0.02	100	100	0.00	0.70
7	0.16	0.02	100	100	0.00	0.04
8	0.14	0.02	100	100	0.00	0.15
9	0.13	0.01	100	100	0.00	0.23
10	0.10	0.01	100	100	0.00	0.46

Where there are three runs, the statistics are calculated by comparing the average of Run-001 and Run-002 against the average of Run-001 to Run-003. In similar fashion, the statistics with six runs are calculated by comparing average of run-001 to run-005 against average of run-001 to Run-006, etc.

The definitions and acceptance criteria taken from DMRB (Volume 12 Section 2) are as follows:

- AAD - The 'Average Absolute Difference' in link flows between successive iterations. This is deemed to be acceptable when less than 1.
- RAAD - The 'Relative Average Absolute Difference' in link flows between successive iterations. This is deemed to be acceptable when less than 1.

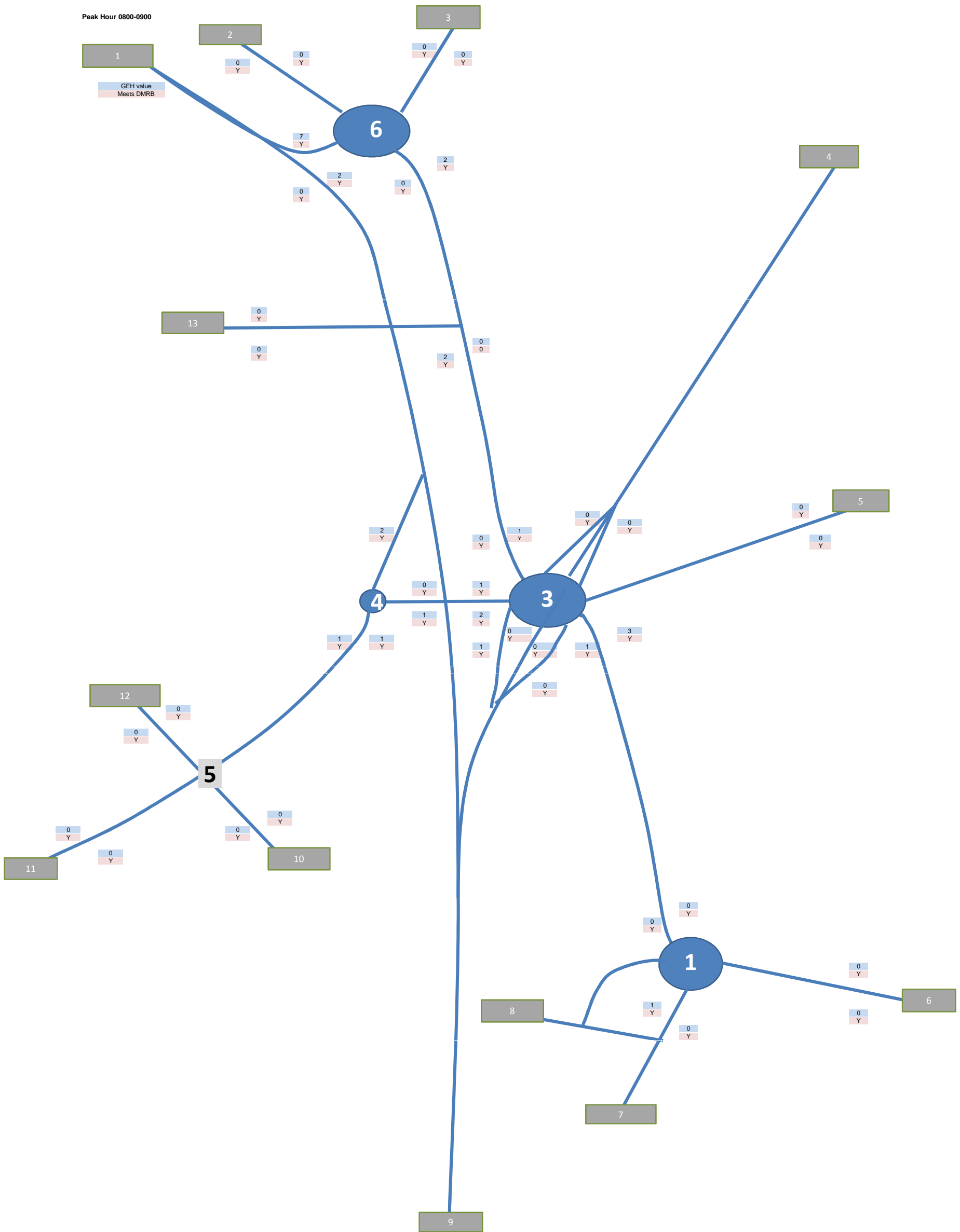
- %FLOW - The proportion of links in the overall network with flows changing less than 5% from the previous iteration. 95% of links must change less than 5% to fit the required criteria.
- STDEV - This is similar to %FLOW. It checks what %age of the current average link flows fall within the previous average +/- the standard deviation of that previous average. The same criterion applies.
- Delta - The Duality Gap, which expresses the flow weighted difference between current total cost estimates on the network and the costs if all traffic would use minimum cost routes as calculated by all-or-nothing assignment. This is deemed to be acceptable when less than 1.
- V - This is the percentage difference in average total travel time between successive runs. It is deemed acceptable when less than 1 for four successive runs.

The analysis shows that the model is stable at approximately 3 runs for both the AM and PM. However, results have been based on a set of 10 runs.

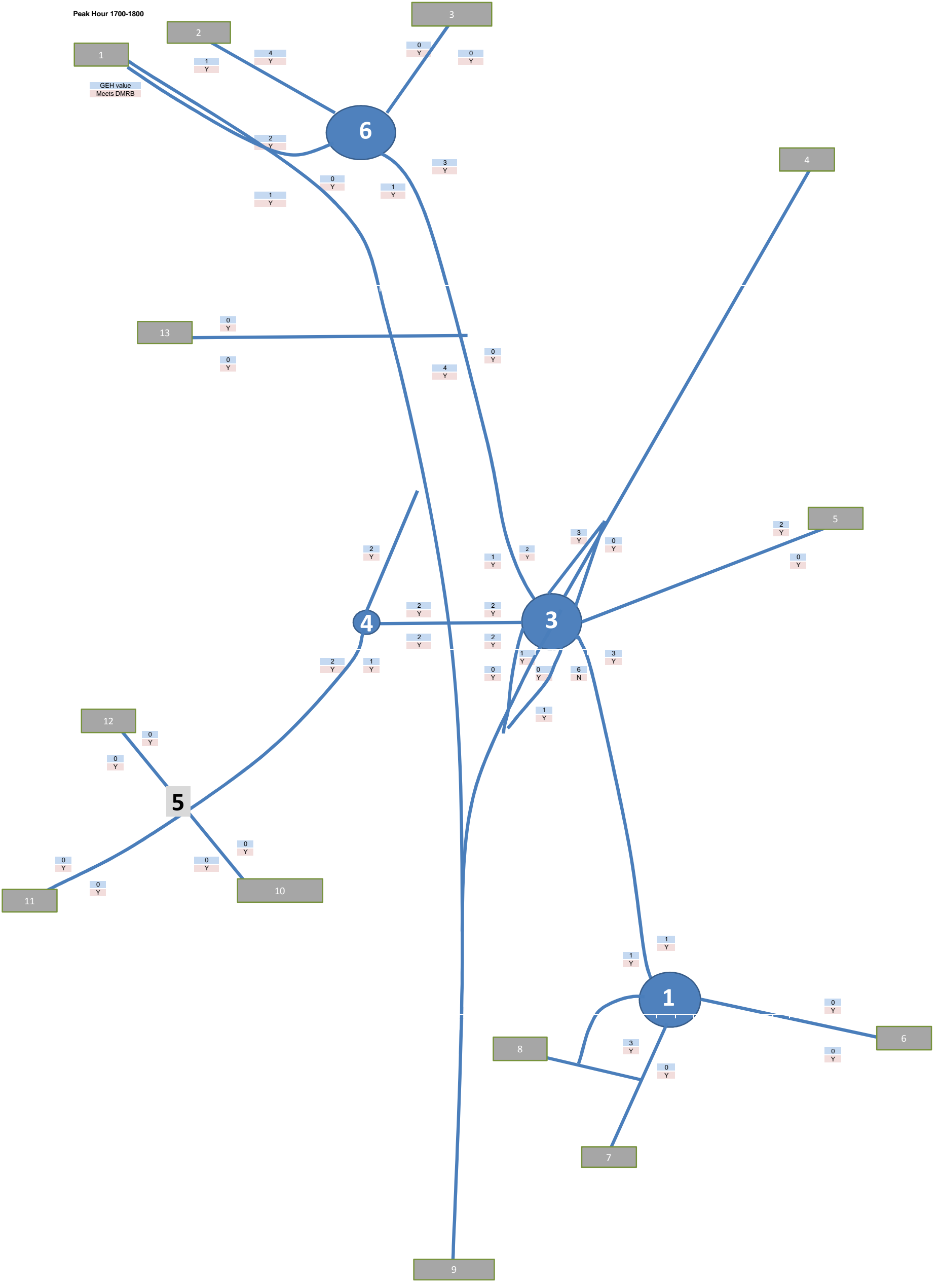
Appendix B

Calibration Results

Peak Hour 0800-0900



Peak Hour 1700-1800



Appendix C

Queue Assessments

Table AppC.1 AM Queue Assessments

		07:30 – 08:00				08:00 – 08:30				08:30 – 09:00				09:00 – 09:30			
Ref.	Approach	Obs (m)	Mod (m)	Difference		Obs (m)	Mod (m)	Difference		Obs (m)	Mod (m)	Difference		Obs (m)	Mod (m)	Difference	
				m	vehs			m	vehs			m	vehs			m	vehs
Site 1	Dunton Ln	8	0	-8	-1	5	0	-5	-1	5	0	-5	-1	5	0	-5	-1
Site 2	Faraday Ave	83	57	-26	-4	78	54	-23	-4	58	44	-14	-2	40	42	2	0
Site 2	A446 (S)	18	30	12	2	28	28	1	0	15	24	9	1	25	23	-2	0
Site 2	Marsh Ln	23	33	11	2	20	33	13	2	13	27	15	2	13	25	13	2
Site 2	A446 (N)	88	68	-19	-3	75	64	-11	-2	100	57	-43	-7	45	42	-3	-1
Site 3	M42 (N)	45	64	19	3	40	63	23	4	58	63	6	1	45	60	15	3
Site 3	Kingsbury Rd (E)	65	108	43	7	58	99	42	7	45	71	26	4	48	65	18	3
Site 3	Lichfield Rd (S)	115	144	29	5	75	107	32	5	85	93	8	1	60	98	38	6
Site 3	M42 (S)	75	94	19	3	63	92	29	5	68	88	20	3	65	90	25	4
Site 3	Kingsbury Rd (W)	65	184	119	20	68	145	78	13	73	102	29	5	33	111	79	13
Site 3	Lichfield Rd (N)	228	219	-9	-1	228	211	-17	-3	175	170	-5	-1	50	133	83	14
Site 4	Kingsbury Rd (E)	0	0	0	0	0	31	31	5	0	14	14	2	0	15	15	3
Site 4	Kingsbury Rd (W)	43	52	10	2	10	32	22	4	13	24	11	2	0	24	24	4
Site 5	Kingsbury Rd (E)	5	0	-5	-1	8	0	-8	-1	5	14	9	2	3	0	-3	0
Site 5	Coleshill Rd	28	13	-14	-2	15	12	-3	0	10	10	0	0	18	21	3	1
Site 5	Kingsbury Rd (W)	20	21	1	0	13	0	-13	-2	10	8	-2	0	5	12	7	1
Site 5	Wishaw Ln	58	35	-23	-4	60	36	-24	-4	60	31	-29	-5	18	32	14	2
Site 6	A4091	68	28	-39	-7	60	23	-37	-6	55	21	-34	-6	35	10	-25	-4
Site 6	Lichfield Rd (S)	55	26	-29	-5	63	30	-33	-5	13	24	12	2	5	27	22	4
Site 6	M6 diverge	30	23	-7	-1	20	28	8	1	13	22	10	2	8	23	15	3
Site 6	Lichfield Rd (N)	105	35	-70	-12	100	52	-48	-8	68	52	-15	-3	73	37	-35	-6

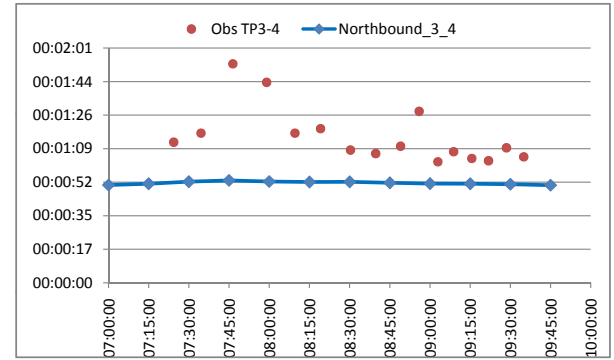
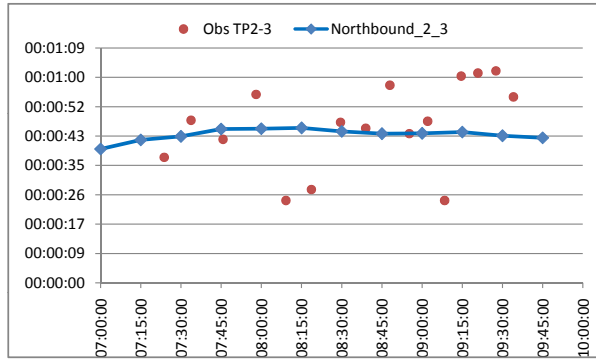
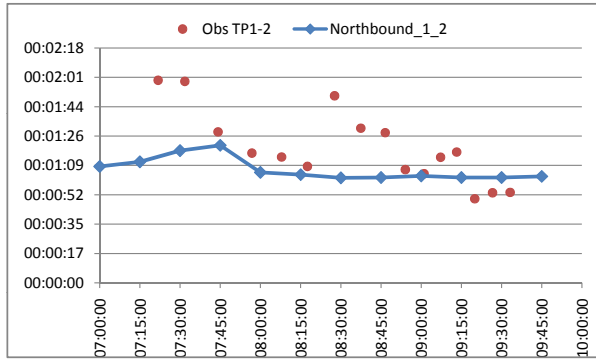
Table AppC.2 PM Queue Assessments

		16:30 – 17:00				17:00 – 17:30				17:30 – 18:00				18:00 – 18:30			
Ref.	Approach	Obs (m)	Mod (m)	Difference		Obs (m)	Mod (m)	Difference		Obs (m)	Mod (m)	Difference		Obs (m)	Mod (m)	Difference	
				m	vehs			m	vehs			m	vehs			m	vehs
Site 1	Dunton Ln	5	15	10	2	5	11	6	1	0	0	0	0	10	0	-10	-2
Site 2	Faraday Ave	63	60	-3	0	65	221	156	26	43	99	57	9	40	55	15	3
Site 2	A446 (S)	23	36	13	2	38	56	19	3	38	44	7	1	35	33	-2	0
Site 2	Marsh Ln	20	29	9	1	18	30	13	2	13	28	15	3	10	26	16	3
Site 2	A446 (N)	18	38	20	3	38	37	-1	0	38	36	-1	0	25	33	8	1
Site 3	M42 (N)	40	64	24	4	53	64	12	2	38	61	24	4	45	64	19	3
Site 3	Kingsbury Rd (E)	43	58	15	3	40	63	23	4	35	61	26	4	38	63	26	4
Site 3	Lichfield Rd (S)	90	112	22	4	78	126	49	8	68	117	49	8	75	95	20	3
Site 3	M42 (S)	123	216	93	16	175	222	47	8	165	210	45	8	148	260	112	19
Site 3	Kingsbury Rd (W)	120	92	-28	-5	75	90	15	2	63	88	25	4	43	78	36	6
Site 3	Lichfield Rd (N)	63	116	53	9	78	193	115	19	88	210	122	20	58	111	54	9
Site 4	Kingsbury Rd (E)	0	16	16	3	0	15	15	3	0	17	17	3	0	14	14	2
Site 4	Kingsbury Rd (W)	25	25	0	0	45	24	-21	-3	28	24	-3	-1	25	22	-3	-1
Site 5	Kingsbury Rd (E)	23	17	-6	-1	28	20	-7	-1	23	16	-6	-1	10	18	8	1
Site 5	Coleshill Rd	45	22	-23	-4	33	24	-8	-1	20	11	-9	-1	20	9	-11	-2
Site 5	Kingsbury Rd (W)	10	7	-3	-1	8	8	0	0	8	7	0	0	13	8	-4	-1
Site 5	Wishaw Ln	10	28	18	3	20	14	-6	-1	13	16	3	1	8	13	6	1
Site 6	A4091	43	20	-22	-4	28	20	-7	-1	28	10	-18	-3	25	11	-14	-2
Site 6	Lichfield Rd (S)	15	31	16	3	33	37	5	1	18	32	15	2	18	35	17	3
Site 6	M6 diverge	8	23	16	3	10	24	14	2	15	24	9	1	10	29	19	3
Site 6	Lichfield Rd (N)	58	85	27	5	70	89	19	3	58	59	2	0	63	73	10	2

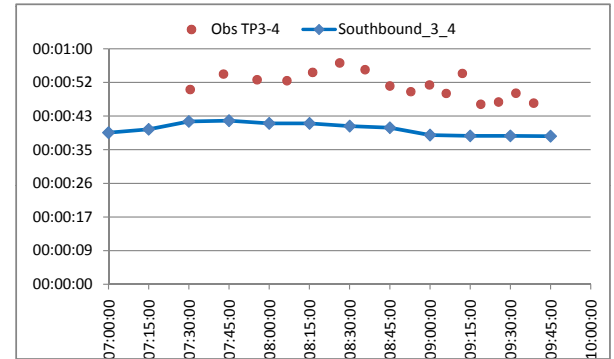
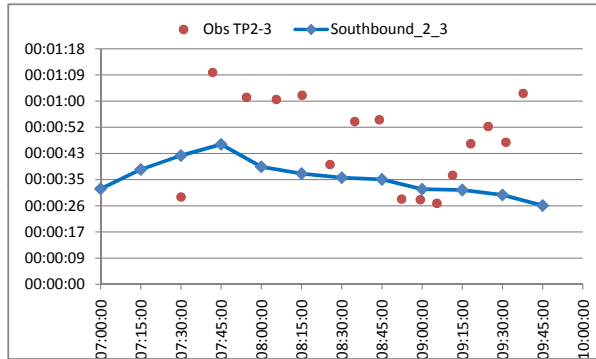
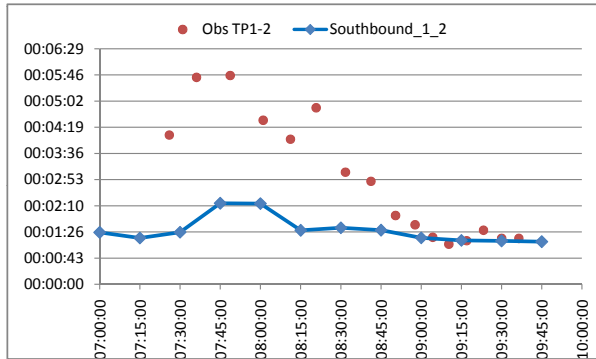
Appendix D

Journey Time Validation

**AM Journey Time Validation
Northbound**

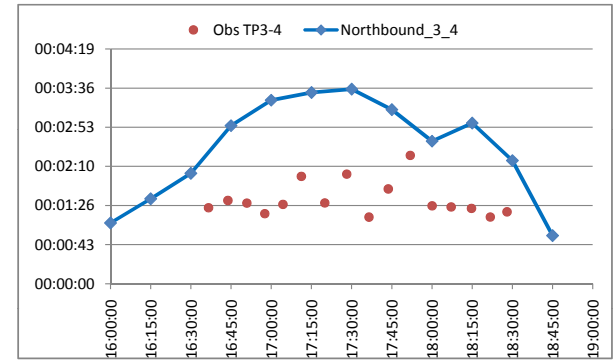
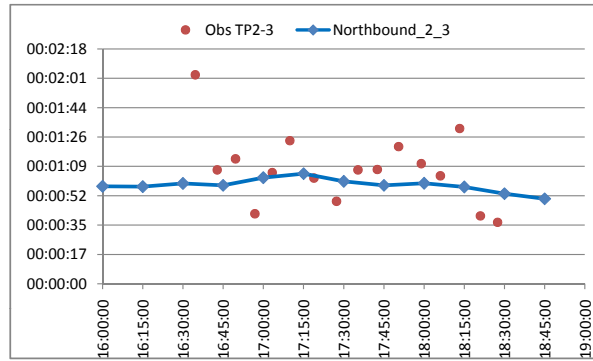
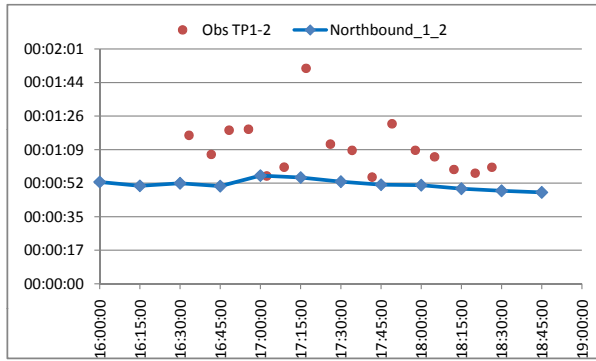


Southbound



PM Journey Time Validation

Northbound



Southbound

